Permeable Pavements in Cold Climates

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Outline

- Introduction and background
- Design methods
- Summary of performance & maintenance
- Case Studies
 - Paired Intersection Study (Minnesota)
 - Porous Asphalt Study (New Hampshire)
 - Woodbridge Neighborhood (Minnesota)
 - Experiences in Colorado
- Summary and Conclusions



THESE PARKING AREAS ARE PAVED WITH DROUS PAVEMEN THAT LEAKS SINCE 1977, IT HAS RAISED THE LOCAL WATER TABLE WHILE REDUCING EROSION, POLLUTION, AND THE NEED FOR STORM DRAINS OR ROAD SALT. A BROCHURE IS AVAILABLE. A DEMONSTRATION PROJECT BY MASS. D.E.P. & MASS. DEM.

Porous pavements developed as early as the 1930s

A sign at a park in Massachusetts.

Image source: MAAPA



Full Depth Permeable Pavement



- Water infiltrates through permeable pavement surface and other layers
- Stored in gravel layer (~40% voids)
- Water infiltrates into soil or is collected by drain tile

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Benefits Permeable <u>Pavement</u>

- Volume reduction
- Improved water quality
- Hydroplaning resistance
- Spray reduction
 - Increased visibility
- Smoother ride
- Noise reduction
- Less salt required





Images source: Barrett 2008



Types of Permeable Pavement

- Porous Asphalt
- Pervious Concrete
- Permeable Pavers
- Permeable Articulated
 Concrete Blocks









Permeable Pavement Design

- No uniform or standard design across industries
- See Weiss et al. (2015) for design recommendations
- Examples of design variations:
 - NAPA: AASHTO design w/ SN. Use non-woven geotextile.
 - ACPA: PerviousPave, uses model developed for StreetPave.
 Use geotextile liner.

- ICPI: AASHTO w/ SN. Geotextile fabric is optional







Keys for Success

- Proper
 Construction
 - Mix design
 - Compaction
 - Void ratio
 - Curing
- Proper and regular maintenance







Summary of Hydraulic Performance

- Surface infiltration rates decrease but are not rate limiting
- Method needed to determine permeability of sub-base before design
- Geotextile fabrics can reduce/eliminate infiltration
- Infiltration rates are maintained through winter

Photo http://ih.constantcontact.com/



Summary of Water Quality Impact

- Removes solids & solid-bound contaminants
- Mass load reduction often through infiltration
- Nitrification may occur (ammonium to nitrate), but total N removal is low
- Dissolved phosphorus removal is minimal







Summary of Maintenance

- Surface cleaning is effective but variable
- Particle removal (top ¼ inch) is major issue
- Pressure washing (45°) and/or vacuuming with regenerative air sweepers is most effective
- Brushes can push material farther into voids
- Clean multiple times per year







Images: Elginsweeper.com

Impact of Vacuuming



Permeable articulated concrete blocks/mats before (A) and after (B) cleaning with a Vac Head.

(Photo courtesy of University of Louisville and D. Buch, PaveDrain, LLC).



Summary of Maintenance

- Major cause of clogging is reduction of surface pavement void space:
 - Heavy loads
 - Particles
 - Lack of maintenance
- No standard to measure or evaluate clogging



Open voids



Partially clogged voids



Porous Asphalt Paired Intersections – Robbinsdale, MN

Constructed 2009-2010



Objective was to evaluate potential salt load reduction on porous asphalt pavements
Also durability, maintenance, and water quality

Construction in September 2010

(Wenck 2014)



- TMDL study for Shingle Creek, MN: Reduce Cl by 81%
- Two porous asphalt pavement intersection constructed: 1) Sand sub-base, 2) Clay sub-base
- Designed for 2-yr storm
- The porous asphalt sections were not salted during the winter
- Conventional asphalt sections were salted







Porous Asphalt Cross-Section

(Wenck 2014)





- Porous asphalt sections: ~150 feet long by ~28 feet wide (4200 square feet)
- Cost: Site 1 was \$42,670

Site 2 it was \$32,200.

 Site 1 construction was negotiated as part of a change order. Site 2 the contract was awarded to the low bidder.



Paired Intersection Study Results





- Winter reservoir temperatures warmer than the pavement temperature
- Reservoir air voids provided insulation



 Insulation minimizes winter freezing and keeps reservoir temperatures cooler in spring



Paired Intersection Study Results



 Suggests winter infiltration into subgrade is possible



 Conventional pavement sites were slushier than the porous asphalt sites due to infiltration into PP



 Bare pavement on the porous test sections comparable to conventional sections but had a lag





Slush gathering and refreezing on the traditional asphalt at Site 1 on January 17, 2010



Slush free porous asphalt on January 17, 2010



Wenck 2014



Site 1 Test Section looking south



Wenck 2014

Paired Intersection Study Lessons Learned





- The unsalted, porous asphalt sections had a similar amount of bare pavement compared to salted, conventional asphalt sections
- The porous pavement over sand subgrade was more effective for ice control compared to the porous pavement on clay subgrade,
 - porous asphalt on sand can infiltrate all or most of the runoff
 - On clay, frequent overflows were observed



Porous asphalt sections have been durable without any special snow plow equipment or adjustments



Paired Intersection Study Lessons Learned





- Effective maintenance on the porous asphalt sections appears to be vacuuming twice per year and patching with traditional asphalt, as necessary
- Porous asphalt intersections have potential as an ice-control management practice in certain situations



UNH Porous Asphalt Parking Lot



Winters of 2006-2007, 2007-2008

Photo from Roseen et al. 2014



	Pervious pavement: 4-6" (10 - 15 cm) of porous asphalt
	Choker Course: 4"-8" (10 – 20 cm) minimum
	Filter Course: 8" - 12" (20 - 30 cm) minimum thickness of subbase (aka. bank run gravel or modified 304.1)
Filter Bla	anket: intermediate setting bed: 3" (8 cm) thickness of $\frac{3}{8}$ " (1 cm) pea gravel
Reservoi frost pr	ir Course: 4" (10 cm) minimum thickness of $3/4$ " (2 cm) crushed stone for rotection. 4-6" (10-15 cm) diameter perforated subdrains with 2" cover
<u> </u>	Optional-Liner for land uses where infiltration is undesirable (e.g., hazardous materials handling, sole-source aquifer protection)

- Each lot = 5000 ft²
- 4" of porous asphalt
- 18% voids, 5.8% asph.
- Filter Course: K=10-60 ft/day at 95% comp.
- Filter blanket prevents migration of fines
- 21 inch stone reservoir
- Underdrain 12 inches above bottom



Image from Roseen et al. 2014



- PA lot received 25% of typical salt load of 3 lb/1000 sf
- DMA received 100% of typical salt load
- Frost penetration
 deeper on PA (27" vs.
 18")
- PA lot thawed ~30 days before DMA
- 25% of runoff infiltrated in PA (Type C soils)

a) PA at 11:20 AM; b) PA at 1 PM; c) DMA at 11:20 AM; d) DMA at 1 Pm

(Roseen et al. 2014)





Lots one hour after plowing (-4° C) (Photo: UNHSC)



- PA exported nitrate;
- PA: no impact on TP;
- PA reduced TPH (1970 μg/L to 166 μg/L

- PA reduced TSS (54 mg/L to 6 mg/L)
- PA mean infiltration rate = 1700 in/hr after 3 yrs & no maintenance



Pavement after freezing rain: a) PA, b) DMA (Roseen et al. 2014)



UNH PA Parking Lot Study-Conclusions

- PA with 25% of salt load had same snow/ice cover as DMA lot
- Salt loads could be reduced by 64% with no compromise in safety
- PA froze but maintained high infiltration capacity
- PA had higher skid resistance (for wet, snow, & compacted snow)
- More salt applications may be necessary
- PA particles were found in voids after winter

Roseen et al. 2014.

Photo: Heather Lynn Peters



Woodbridge Neighborhood-Shoreview, MN



Pervious Concrete, constructed in 2009.



Woodbridge Neighborhood

Initially:

- 38 ac, fully developed
- 9000 yd² of asphalt
- Storm drainage concerns

Needed to:

- Replace road, upgrade utility, improve stormwater management
- Total cost = \$15M





Woodbridge Neighborhood



Why PC?

- Free draining soils
- Advances in mix designs and placement techniques
- Same cost as conventional asphalt with storm drains

Project construction.



Woodbridge Neighborhood -Construction

18" crushed rock reservoir

- Tri-roller screed for consolidation
- Curing fabric used instead of poly sheeting placed within 1 minute (7 day duration)
- Mix Design: 125 PCF, 21% air voids (+/- 3%)
- 7" of pervious concrete
- 1.5" Railroad ballast, 18-30" thick
- \$86.30 per SY
- Saw cut joints 24-48 hours after pour

Curing of Pervious Concrete.



Woodbridge Neighborhood -Maintenance

- Regenerative air sweeper (no brushes); ~ every 6
 weeks
- No salt or sand application
- Plowed by one-ton pickup w/ regular plow
- Clogging occurs mostly in top ¼" of pavement
- Maintenance has maintained infiltration rates of 300-500 in/hr in most areas

Project Construction.



Lessons Learned

- Construction & curing very important
- Saturated curing blankets have been successful
- Saw cut joints have been successful



Saw cut joint. Photo courtesy of M. Maloney



Lessons Learned

- Reservoir aggregate should be large & angular
- Salt and turning traffic have caused isolated failure
- Organics are the main source of clogging
- Do not work PC by hand
- Do not "walk" screed around corners



1.5" Railroad ballast. Photo: Florence Crushed Stone



The Denver (UDFCD) Experience

Denver, Colorado. Photo: PlaneandJane.com



Aurora Wal-Mart Parking Lot

- Pervious
 Concrete
- Installed in 2004
- No info on mix design
- Raveling at joints (some saw cut)





Denver Safeway Parking Lot

- Pervious
 Concrete
- Installed in 2004
- No info on mix design
- Surface erosion





University Plaza Parking Lot

- Pervious
 Concrete
- Installed in 2005
- No info on mix design
- Surface erosion





Possible Causes of Failure

- Non-uniform void content
- Poor air entrainment in cement paste
- Chloride (applied/carried in)
- Cement paste consolidation
- Placement during adverse weather
- Loss of hydration water during curing



Close up of parking lot stall. Photo courtesy of K. MacKenzie, UDFCD



National Renewable Energy Lab Parking Lot

- Pervious
 Concrete
- Installed in 2009
- Mix design followed new requirements
- PC use in Denver suspended





Denver Waste Management Building

- Porous Asphalt
- Installed in 2008
- Surface infiltration < 20 in/hr
- Intensive maintenance was ineffective



Parking lot after light rain. Photo courtesy of K. MacKenzie, UDFCD



Denver Waste Management Building

- Cores revealed proper construction (17% voids, proper PSD, asphalt content, etc.)
- More than half of other PA sites have infiltration
 < 20 in/hr
- UDFCD does not recommend use of PA



Parking lot after snowfall. Photo courtesy of K. MacKenzie, UDFCD





- Permeable pavements can result in less winter salt application
- Permeable pavements can reduce runoff volume and improve water quality (with other benefits)
- Permeable pavements are more expensive to construct
- Construction & maintenance are critical to success



Conclusions (Cont'd)

- Maintenance: pressure washing and/or vacuuming
- Permeable pavements can withstand harsh winters
- Permeable pavements can maintain infiltration rates throughout the winter



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Questions?

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Photo: http://itcontrolsfreak.files.wordpress.com/2012/11/rain1.jpg



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Porous Asphalt Design Recommendations

- 1. Particle size distribution & binder type are the 2 most important factors in mix selection (Jones et al. 2010; Li et al. 2012)
- Void ratios of 25% & infiltration rates of > 7 cm/s possible by optimizing aggregate (Partl 2003)
- 3. Typical air voids are 16 20% (NAPA 2008)
- 4. Depth of aggregate bed to be 65% of frost depth (UNHSC 2009)
- 5. Typical aggregate gradations/specs given in reports (NAPA 2008)



Pervious Concrete Design Recommendations

- 1. Course aggregate: 3/8 3/4 inch. All aggregates meet ASTM D448 and C33/C33M (ACI 2010)
- 2. Low water:cement ratios (i.e. 0.26-0.34)
- 3. Supplementary materials (e.g. fly ash)/admixtures may be used (must meet ASTM requirements)
- 4. Void content from 15-25%
- 5. Increase in sand content may increase freeze-thaw resistance (CRMCA 2009)



Permeable Paver Design Recommendations

- Open-graded bases: <2% fines, density: 95-120 lb/ft3, porosities >30%
- 2. All stone & aggregate: >=90% fractured faces and a minimum Los Angeles abrasion value of less than 40
- 3. Base and sub-base: porosity >= 32%, CBR >= 80%

